



Design and Analysis of Stone Column

Vaishali Pali, Department of Civil Engineering, AISECT University, Bhopal
Kapil Soni, Department of Civil Engineering, AISECT University, Bhopal

Abstract

The present work portrays the limited component examination completed to contemplate the impact shear quality of soil, distance across proportion, outer support by assaulting the stone segment and inside fortification by giving level round strips on the protruding conduct of stone segment and load conveying limit. In this paper proposed work for to increase the bearing capacity of the ground. It also implemented it on a village road where generally black cotton soil available.

ISSN 2454-308X



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Keywords: Soil Strength improvement, Geosynthetics, Vibrocompaction, Grouting, Compaction, Stone Column vertical

1. Introduction

Ground improvement techniques are given the utmost importance in present days to adapt weak ground/soil into the appropriate competent stable ground for different civil engineering applications.

It began in (1960) and got comfortable with the pioneer work of Binquet and Lee. Ground improvement methods are suggested in troublesome ground conditions as mechanical properties are not sufficient to endure the superimposed load of framework to be manufacture, swelling and shrinkage property more articulated, collapsible soils, delicate soils , natural soils and peaty soils, karst stores with sinkhole arrangements, establishments on dumps and clean landfills, Handling care of dug materials for establishment beds, taking care of unsafe materials in contact with soils, utilizing of old dig pits as site for proposed foundation. At the point when an undertaking site run over any of the above difficult conditions, conceivable elective arrangements might be one of among as maintain a strategic distance from the specific site; outline the arranged structure (adaptable/inflexible) in like manner, expel and supplant inadmissible soils, endeavor to alter existing ground, empower financially savvy establishment configuration, lessen the impacts of sullied soils, guarantee supportability in development ventures utilizing ground change systems. While it may not be instantly evident, ground change strategies have made significant advances since the present normally

rehearsed systems started to create in the twentieth century anyway most methods have experienced changes. This paper introduces an audit on innovative work in the field of ground change. (1).

Presence of inadmissible soil for supporting structures in building locales, absence of space and financial inspiration are essential fundamental purposes behind utilizing soil change procedures with poor subgrade soil conditions instead of profound establishment. A few techniques are regularly used to lessen the post development settlement, upgrade the shear quality of the soil framework, increment the bearing capacity of the soil, and enhance the steadiness of dams and embankments. (2)

Expressed that soil improvement techniques can be isolated into four fundamental classifications:

1. Soil improvement without admixtures (soil replacement, preloading, sand drains, vertical drains)
2. Soil improvement with admixtures or inclusions (stone columns, sand compaction piles)
3. Soil improvement using stabilization with additives and grouting methods (chemical stabilization, deep mixing, jet grouting)
4. Soil improvement using thermal methods (heating & freezing)

2. Methodology

In this proposed work following steps are taken to study the parameters are as follows:

- i. Research work done in past related to ground improvement technique is studied and determine the future scope to be implement in this study.
- ii. Further sample preparation and procedure to apply it on ground is considered.
- iii. In the very first step samples are installed with grouting done of fly ash at varying depth to study the results.
- iv. After that experimental tests are done to justify the variation and outcome of our study.
- v. Than after we conclude our study by providing our outcome and its beneficial in future research.

MATERIAL USED:

The Mohr-coulomb investigation requires aggregate of these six parameters.

These parameters are

- Young's modulus (E)
- dry unit weight (γ_d)
- Poisson's proportion (μ)
- point of inside rubbing (ϕ)
- Unit undrained attachment (c_u)
- dilatancy point (ψ).

The info parameters (E, μ , ϕ , ψ , c_u , γ_d) are given in Table 1.

Table 1 parameters of selected soil

Material	W (%)	E (kPa)	μ	c_u (kPa)	Ψ (deg)	Φ (deg)	γ_d (kN/m ³)	γ_{sat} (kN/m ³)
Clay	25	5500	0.42	30	-	-	15.56	19.45
	30	3100	0.45	14	-	-	14.60	18.98
	35	2150	0.47	7	-	-	13.60	18.38
Stone	-	55000	0.30	-	10°	44°	16.62	-

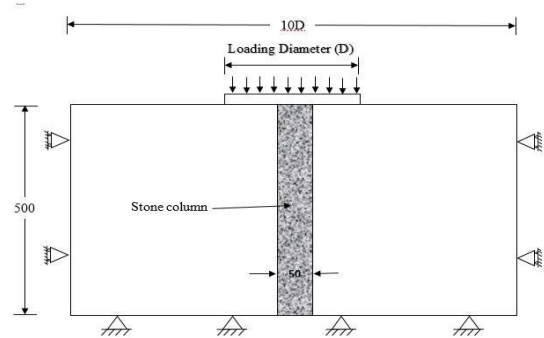
2.1 TEST-1: EFFECT OF DIAMETER RATIO

With variety in measurement proportion there are varieties in parameters of stone segments. Hence an entire model of stone section is worked to dissect the impact of distance across proportion on:

- a) Ultimate quality of stone segment
- b) Depth of greatest protruding
- c) Diameter of greatest protruding.

A schematic view of loading plan on a stone column confined by soft clay is shown in Fig.1

Fig.1 A schematic view of loading plan
2.2 TEST-2: EFFECT OF LENGTH OF CIRCUMFERENTIAL ENCASEMENT



One technique to enhance the execution of stone segments introduced in delicate soils is wrapping the general stone section remotely with an appropriate geosynthetic in a tubular frame. This kind of encasement by a geogrid or geotextile grants extra parallel restriction and makes the stone segments stiffer and more grounded. In this way an entire model of stone section is worked to dissect the impact of circumferential encasement on:

- a) Ultimate quality of stone section
- b) Position of protruding

A schematic view of loading plan on an encased stone column confined by soft clay is shown in Fig.2.

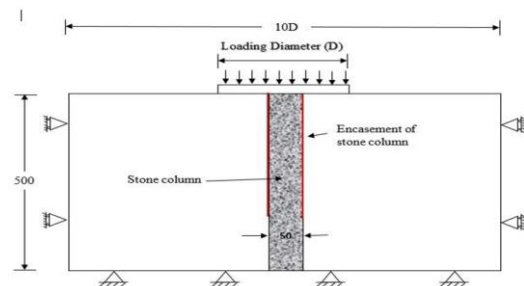


Fig.2 A schematic view of loading plan of encased stone column

2.3 TEST-3: EFFECT OF HORIZONTAL CIRCULAR STRIP

Another technique for enhancing the execution of stone sections is by giving level roundabout strips inside at consistent interim to confine the protruding. Subsequently a total model of stone section is worked to break down the impact of flat roundabout strip on extreme quality of stone segment. A schematic

perspective of stacking plan of a stone section with level round strips kept by delicate earth is appeared in Fig. 3.

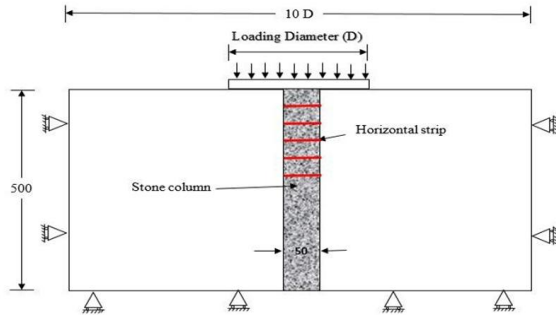


Fig.3 A schematic view of loading plan of stone column with horizontal circular strips

2.4 TEST-4: EFFECT OF HORIZONTAL CIRCULAR STRIP ON ENCASED STONE COLUMN

With a specific end goal to upgrade the execution of stone section, a mix outside fortification and interior support is considered. A schematic perspective of stacking plan of an encased stone section with flat roundabout strips kept by delicate dirt is appeared in Fig. 4.

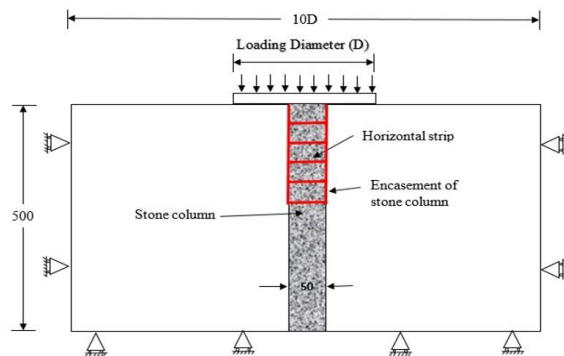


Fig.4 A schematic view of loading plan of encased stone column with horizontal circular strips

3. Result and Discussion

A stone section infers its heap conveying limit from the constraintment offered by the encompassing soil. Encasement of stone section has been expanded the utilization of stone segments to delicate dirt. The present investigation contains stone segment with three sort of additional repression circumferential encasement, round flat strips and mix of two. An axisymmetric examination was completed utilizing Mohr-Coulomb's model considering elasto-plastic conduct for delicate dirt and stone. Three sort of encompassing is thought about 7, 14, 30 kPa and the heap conveying limit of footings situated over stone segments is contrasted and measure up to size of footings situated on the virgin soil that is without a stone section underneath.

3.1. RESULT OF TEST-1

Effect of Diameter Ratio on Ultimate Strength

By changing the distance across (proportion of breadth of stacking region and measurement of stone section), the adjustment in a definitive quality is watched for the diverse quality of the keeping material. Fig. 5 demonstrates the connection between the distance across proportions to extreme quality for the diverse shear quality ($c_u = 7\text{kPa}$, 14kPa and 30kPa) of keeping soil. From the figure, plainly there is a little change in extreme quality of stacked territory after measurement proportion of 5. Table 2 demonstrates a definitive quality and the rate augument in most extreme quality for various distance across proportion for various imprisonment shear pressure c_u . There is 10 % augument in extreme worry for breadth proportion of 6 as for virgin mud however it can be expanded up to 100 % by keeping the protruding using any and all means i.e. by giving geosynthetic encasement or geogrid even strip.

Table 2 Effect of diameter ratio on ultimate strength of stone column for different c_u

Diameter Ratio (Dr)	Cu=7 kPa		Cu=14 kPa		Cu=30 kPa	
	Ultimate Strength (kN/m ²)	Increment in Strength (%)	Ultimate Strength (kN/m ²)	Increment in Strength (%)	Ultimate Strength (kN/m ²)	Increment in Strength (%)
1	208	365	397	346	816	332
2	86.4	93	167.2	88	348	84
3	63.3	41.6	124.1	39.4	261	38
4	54.4	21.7	107.6	21	227	20
5	51.2	14.5	101.4	14	213.7	13
6	50.0	11.8	98.7	11	208.4	10
7	47.8	6.9	94.8	6.5	202	7
8	46.8	4.7	93.0	4.5	197.5	4.6

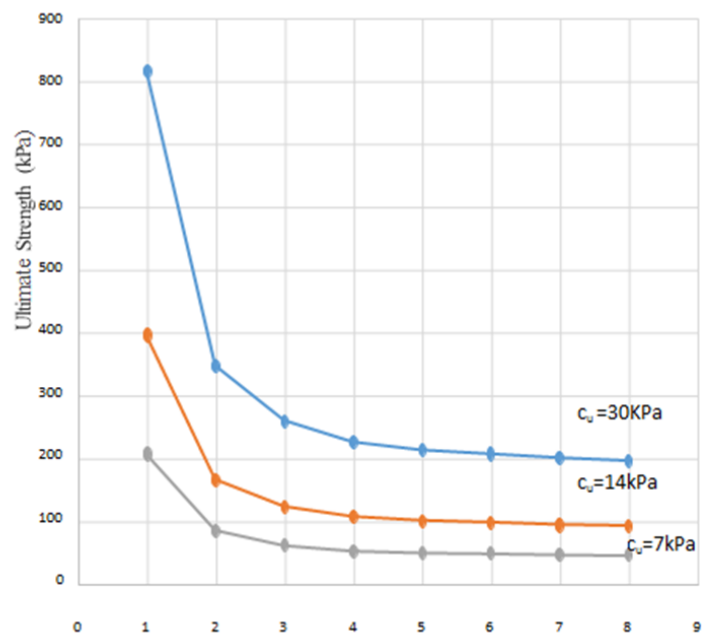


Fig.5 Effect of diameter ratio and c_u on ultimate stress of stone column



4 Conclusion

In view of the outcomes got from this examination the accompanying conclusions are made:

- For low quality of soil (delicate soil), as geogrid encasement length expands, a definitive quality of stone section increments. In spite of the fact that, the rate of increment of extreme quality declines with expanded encasement length. Be that as it may, for high quality of soil (hardened soil), half-way encasement in upper segment of stone section is more compelling.
- For delicate soil, as number of even round strips expands a definitive quality increments and discovered support over the full section length gives higher extreme quality be that as it may, for hardened soil, fortification in upper locale is successful.
- Combination of outer fortification (circumferential encasement) and inward support (level roundabout strips) is more successful in firm soil as opposed to delicate soil.

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